

**TIN CUP CREEK
INUNDATION STUDY FOR
THE AREA EAST OF "REACH 8"**
September 7, 2001

GENERAL SCOPE:

The purpose of this study is to produce an inundation map for the lower reaches of Tin Cup Creek that will delineate an estimate of the outer perimeter of the area that would be impacted by a "Clear Weather" breach of Tin Cup Dam. Tin Cup Dam is located approximately 13 miles upstream from the study area.

Prior to this study a breach analysis for Tin Cup Dam was concluded by Laurence Siroky, P.E. of the Montana Department of Natural Resources and Conservation. This DNRC study developed breach flows for the dam and modeled the flood flows from the dam to a point on Tin Cup Creek identified as "Reach 8", which is just upstream from an existing bridge on Highway 93 near the town of Darby. Our study is limited to the area east of "Reach 8" of the DNRC model and extends to the Bitterroot River, which is the terminus of Tin Cup Creek. As directed by the USFS and the Tin Cup Water District we have taken the breach flows for our analysis directly from the DNRC report . Our study does not attempt to validate the DNRC model but simply considers the affects of the predetermined breach flows on the study area.

HYDRAULIC STUDY:

I. General

Two general breach scenarios were considered for this study. The first case was to consider breach flows with the dam at normal full pool elevation, approximately equal to the top of the existing spillway, which is elevation 6284.3. The flow at "Reach 8" taken from the DNRC model for this scenario is 5,255 cfs. The second case represented a breach of the dam while a temporary dike was in place raising the dam pool elevation to 6286.3. The DNRC model flow at "Reach 8" for this stage was given as 6,326 cfs.

II. Site Survey

Six cross sections were surveyed between "Reach 8" of the DNRC analysis and the Bitterroot river. The locations of these sections are shown on Exhibit A of this report. All sections were tied to USGS NAVD 88 datum.

III. HEC-RAS Analysis

A HEC-RAS computer model was developed for the lower reaches of Tin Cup Creek using the surveyed data and flows provided by the DNRC. As a method of calibration, water surface profiles were computed using the flows for the 2 year (Q2) and the 100 year (Q100) reoccurrence intervals. The computed water surface elevations and stream depths for the Q2 event were compared to the "bank full" widths and channel depths from the surveyed cross sections and for the Q100 event the computed water surface at the Highway 93 crossing was compared to the elevation for the same flow computed by the Montana Transportation Department as shown on the bridge plans (Federal Aid Project No. RTF-BRF 7-1(36)23, dated 5-19-92). Both computed profiles produced favorable results and our model was assumed valid for the purpose of this study.

DISCUSSION:

The HEC-RAS analysis highlighted two major points of constriction to the anticipated breach flows, the first being the Highway 93 bridge over Tin Cup Creek and the second consisting of two existing adjacent bridges over Tin Cup Creek located on Water Street approximately 1,000 feet downstream from the Highway 93 crossing. Inundation areas were developed assuming that these structures, as well as the roadbed, remain in place during a flood event.

In determining the inundation area we assumed that the sharp bend the natural channel makes just before reaching Highway 93 directly east of "Reach 8" will act to dissipate some of the energy and velocity of the breach flows. For this reason we believe it is possible that flows will back up behind the bridge and would not cross over the highway in any significant amount before filling all low lying areas west of Highway 93. We have calculated that approximately 3,500 cfs can pass through the existing bridge opening at the Highway 93 crossing before overtopping the roadway. The most significant difference between the two breach flow cases considered (Case 1- Pool at the spillway crest and Case 2- Pool 2' above the spillway crest) would be the depth of water impounded in the area west of Highway 93. We estimated that the larger flow associated with the higher pool elevation will result in approximately 40% more water being impounded behind the Highway 93 bridge than for the smaller flow associated with the normal full pool elevation. This larger volume of stored water increases the probability that some localized flow across Highway 93 may occur at the existing low point in the road near the present Sinclair gas station at the south edge of Darby.

The restriction of the two downstream Water Street structures is much more significant than that of the Highway 93 bridge. These bridges only have capacity to carry approximately one fourth of the anticipated breach flows (1,350 cfs) before overtopping the roadway. The area between Highway 93 and these structures is a broad flood plain. Once flows exceed the capacity of the natural channel and water overtops the existing creek banks east of Highway 93 we

anticipate that water would travel overland as sheet flow and follow the natural downhill grade to the river.

In addition to the area that would be directly affected by the flooding mechanism outlined above we have also considered the possible impact from the breach flows exiting the natural, relatively narrow, constriction of the canyon just upstream from "Reach 8". It is our opinion that a strong "surge" flow would hit the highway before making the existing 90 degree river bend sending a percentage of the flow over Highway 93 at this point. As can be seen in the attached aerial photo (Exhibit A) the direct distance from the mouth of the canyon near "Reach 8" east to the Bitterroot River is approximately 1400 feet, this is compared to the 5000 feet which is the distance traveled along the current creek channel to the Bitterroot River. Highway 93 is the only barrier preventing the breach flows from taking this direct line to the river making it very probable that some water will cross the highway at least for a short period of time. This would also be a likely location for debris being carried by the breach flows to be deposited, possibly altering the course of the flow, and would be another reason to include this area in the inundation map.

CONCLUSION:

Due to the numerous man-made impediments to the natural course of Tin Cup Creek, such as, the Highway 93 road fill, bridges, buildings, etc., and the accompanying potential for drastic impacts of debris to the possible course the flood flows take, it is difficult to use only conventional modeling approaches to determine the boundary of the inundation area for a "Clear Weather" breach of Tin Cup Dam. It is our opinion that the inundation area we have shown (Exhibit B) provides a reasonable estimate of the area that could be impacted by breach flows and can be used as a guide to develop evacuation and warning plans and procedures.

In determining the inundation area no attempt has been made to differentiate between areas of severe flooding (greater than 2' in depth) and minor flooding (6" or less in depth) but only to delineate an estimate of the outer perimeter of the area that would expect to be flooded by a "Clear Weather" breach of the dam.



Mike Jensen, P.E.
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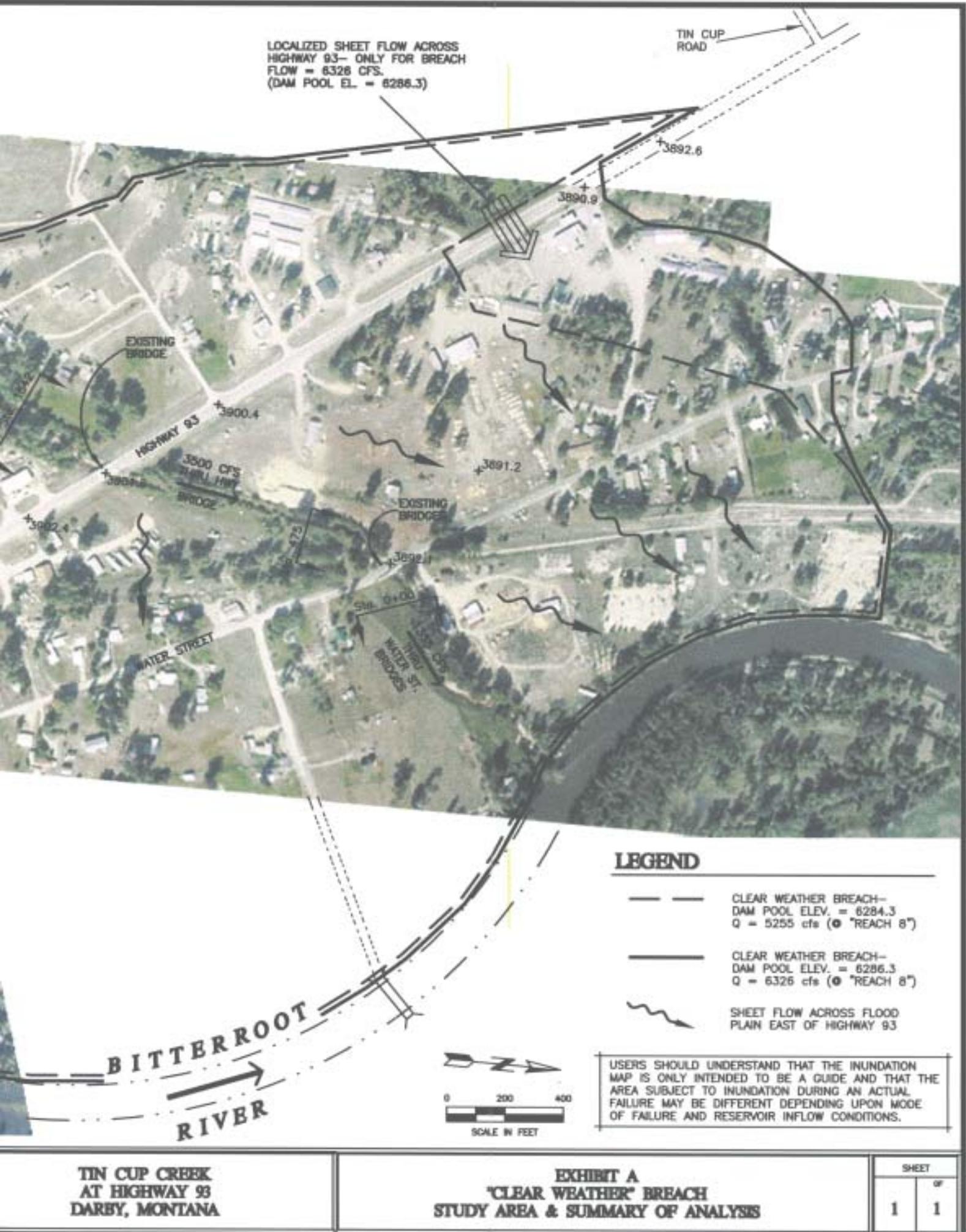


BY	DATE	REVISION DESCRIPTION

DESIGN MU PROJ. NO. 31138
DRAWN MU DATE 8/21
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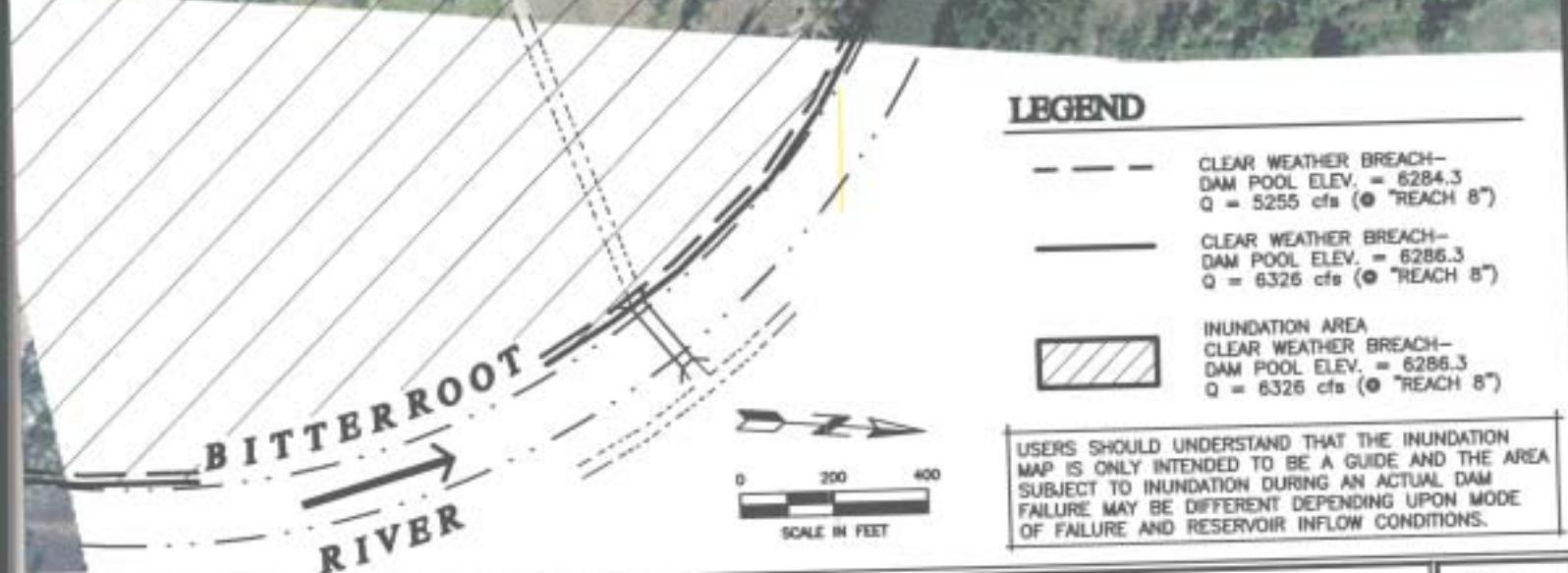


BY	DATE	REVISION DESCRIPTION

DESIGN MJL PROJ. NO. 4114a
DRAWN MJL DATE 5/21
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LEGEND

— — — CLEAR WEATHER BREACH—
DAM POOL ELEV. = 6284.3
Q = 5255 cfs (• "REACH 8")

— — — CLEAR WEATHER BREACH—
DAM POOL ELEV. = 6286.3
Q = 6326 cfs (• "REACH 8")

INUNDATION AREA
— — — CLEAR WEATHER BREACH—
DAM POOL ELEV. = 6286.3
Q = 6326 cfs (• "REACH 8")

USERS SHOULD UNDERSTAND THAT THE INUNDATION MAP IS ONLY INTENDED TO BE A GUIDE AND THE AREA SUBJECT TO INUNDATION DURING AN ACTUAL DAM FAILURE MAY BE DIFFERENT DEPENDING UPON MODE OF FAILURE AND RESERVOIR INFLOW CONDITIONS.

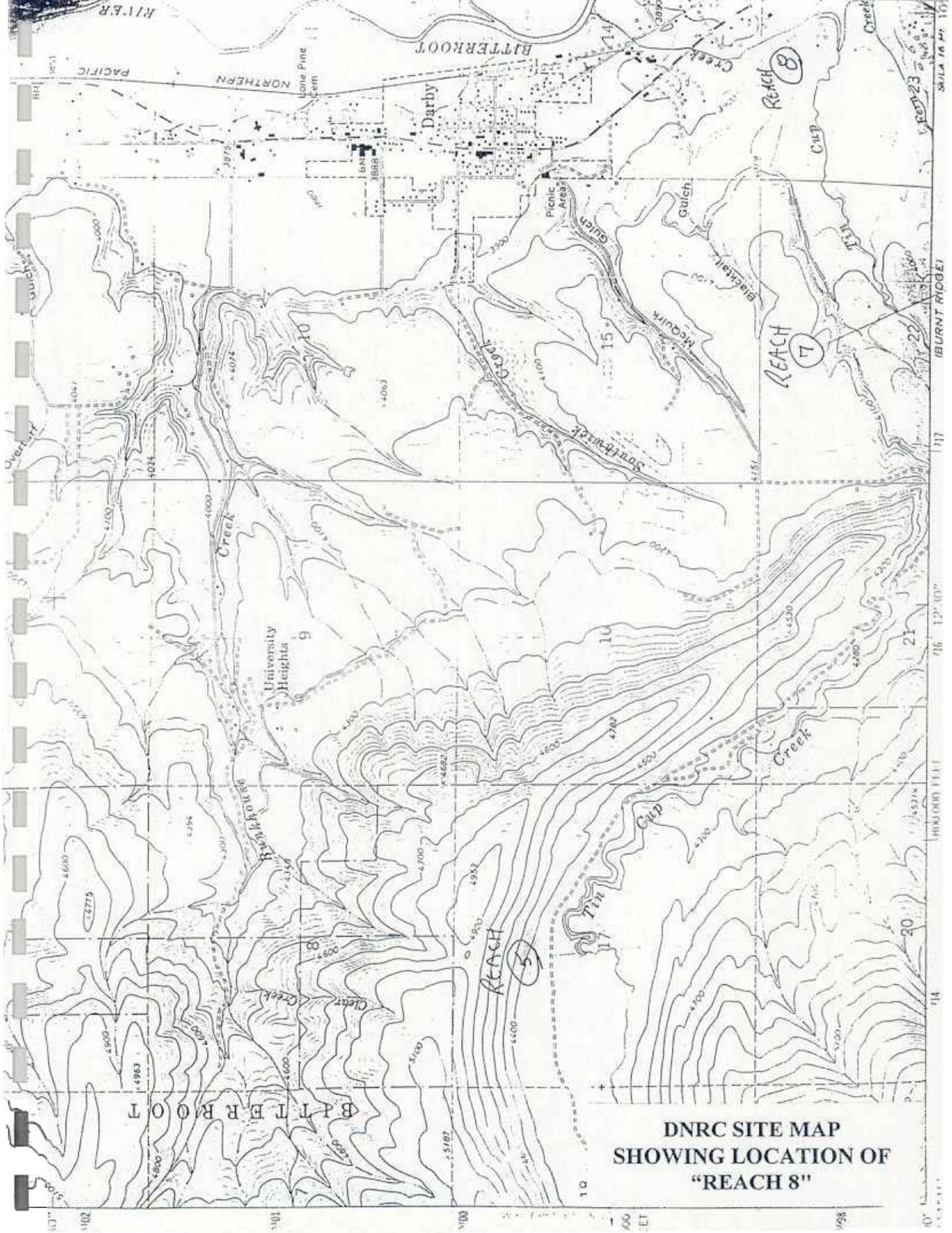
TIN CUP CREEK
AT HIGHWAY 93
DARBY, MONTANA

EXHIBIT B
"CLEAR WEATHER" BREACH
INUNDATION AREA EAST OF "REACH 8"

SHEET
1 OF
1

BREACH ANALYSIS
TIN CUP DAM
APRIL 2000
DNRC
L. SIROKY

NORMAL RESERVOIR ELEVATION	BREACH WIDTH (FEET)	BREACH TIME (HR)	BREACH FLOW @ DAM	BREACH FLOW @ REACH 8	BREACH ELEVATION @ REACH 8	FILE REFERENCE NAME
6283.3	25	.3	2453	1847	3893.8	TIN8325.HOU
6283.3	50	.3	4319	3041	3895.3	TIN8350.HOU
6283.3	75	.3	6144	4082	3896.3	TIN8335.HOU
6284.3	75	.3	7353	5255	3897.5	TIN8435.HOU
6285.3	75	.3	8663	5529	3897.7	TIN8535.HOU
6286.3	75	.3	10049	6326	3898.4	TIN8635.HOU
6287.3	75	.3	11473	7208	3899.0	TIN8735.HOU
6288.3	75	.3	12432	7712	3899.5	TIN8835. ⁷⁵ HOU
6288.3	75	.5	11914	7652	3899.4	TIN8875.HOU
6288.3	150	.4	21275	11193	3901.7	TIN88.HOU
6293.3	75	.5	18565	12028	3902.2	TIN9375.HOU
6293.3	252	.5	42797	21010	3906.2	TIN93.HOU
6295.3	75	.5	21611	14230	3905.8	TIN9575.HOU
6295.3	300	.5	52951	25412	3907.7	TIN95.HOU



HEC-RAS Plan: w/ bridges; 3 River; 1in cup; Reach; hincup

Reach	River Sta	Q Total	Mn Ch El	W.S. Elev	Max Ch Depth	E.G. Crit W.S.	E.G. Elev	E.G. Slope	Vel Chnl	Flow Area	Top Width Act	Froude # Chl
		(cfs)	(ft)	(ft)	(ft)	(ft)	(ft)	(ft)	(ft/s)	(sq ft)	(ft)	
Reach 3623	3623	160.00	3908.47	3910.41	1.94	3910.41	3910.90	0.0190223	5.64	28.38	29.59	1.01
hincup 3623	4360.5*	1355.00	3908.47	3913.41	4.94	3913.41	3914.87	0.0101443	9.81	159.12	68.66	0.91
hincup 3623	4360.5*	2000.00	3908.47	3914.45	5.98	3914.45	3916.13	0.008722	10.77	237.76	88.31	0.88
hincup 3623	4360.5*	3500.00	3908.47	3915.51	7.04	3915.51	3915.75	0.002122	6.10	1529.58	479.36	0.45
hincup 3623	4360.5*	4002.00	3908.47	3915.51	7.04	3915.51	3915.84	0.002880	7.11	1529.58	479.36	0.53
hincup 3623	4360.5*	5529.00	3908.47	3915.51	7.04	3915.51	3916.11	0.008295	9.63	1529.58	479.36	0.71
hincup 3623	4360.5*	6326.00	3908.47	3915.51	7.04	3915.51	3916.30	0.006632	11.02	1529.58	479.36	0.81
hincup 3623	4360.5*	160.00	3905.60	3906.70	1.10	3906.70	3906.87	0.005380	3.25	50.16	47.96	0.55
hincup 3623	4360.5*	1355.00	3905.60	3907.08	2.08	3908.64	3910.79	0.045994	14.21	105.16	132.19	1.79
hincup 3623	4360.5*	2000.00	3905.60	3908.05	2.45	3909.26	3912.22	0.052446	16.81	154.45	133.12	1.96
hincup 3623	4360.5*	3500.00	3905.60	3908.86	3.26	3910.34	3913.98	0.046942	19.55	262.85	135.14	1.99
hincup 3623	4360.5*	4002.00	3905.60	3909.32	3.72	3910.75	3913.90	0.039391	18.84	325.87	136.30	1.80
hincup 3623	4360.5*	5529.00	3905.60	3910.76	5.16	3911.07	3913.93	0.019010	16.27	535.60	170.17	1.32
hincup 3623	4360.5*	6326.00	3905.60	3911.07	6.07	3912.13	3914.22	0.012483	14.85	709.72	209.18	1.10
hincup 3623	4360.5*	160.00	3902.35	3903.65	1.30	3903.65	3904.07	0.019037	5.17	30.95	38.08	1.01
hincup 3623	4360.5*	1355.00	3902.35	3905.92	3.57	3905.92	3907.12	0.013639	8.79	154.10	67.31	1.00
hincup 3623	4360.5*	2000.00	3902.35	3906.90	4.55	3906.90	3907.91	0.008177	8.41	318.64	215.09	0.81
hincup 3623	4360.5*	3500.00	3902.35	3909.15	6.80	3909.57	3909.94	0.002294	6.28	1085.58	396.07	0.47
hincup 3623	4360.5*	4002.00	3902.35	3909.78	7.43	3910.16	3911.67	0.001675	6.10	1342.37	412.22	0.43
hincup 3623	4360.5*	5529.00	3902.35	3911.10	8.75	3911.44	3912.28	0.001426	6.05	1907.85	447.46	0.39
hincup 3623	4360.5*	6326.00	3902.35	3911.71	9.30	3912.04	3913.10	0.001310	6.11	2182.92	457.25	0.38
hincup 3623	4360.5*	160.00	3900.20	3901.47	1.27	3901.47	3901.61	0.003936	3.00	63.41	44.05	0.48
hincup 3623	4360.5*	1355.00	3900.20	3904.53	4.33	3904.53	3905.22	0.004456	6.67	203.20	53.80	0.60
hincup 3623	4360.5*	2000.00	3900.20	3905.56	5.36	3905.56	3906.40	0.004657	7.09	280.11	57.07	0.63
hincup 3623	4362.3	3500.00	3900.20	3907.19	6.99	3908.68	3909.625	0.005625	9.73	357.49	62.27	0.72
hincup 3623	4362.3	4082.00	3900.20	3907.02	7.42	3906.47	3909.32	0.005917	10.53	428.36	171.00	0.75
hincup 3623	4362.3	5529.00	3900.20	3909.31	8.11	3908.31	3910.62	0.007145	12.43	548.63	171.00	0.84
hincup 3623	4362.3	6326.00	3900.20	3909.84	8.64	3908.84	3911.25	0.006946	12.67	638.39	171.00	0.83
hincup 3623	4362.3	160.00	3899.43	3900.69	1.26	3900.69	3900.82	0.003683	2.86	55.98	47.20	0.46
hincup 3623	4365.0*	1355.00	3899.43	3903.66	4.23	3904.30	3904.276	6.47	209.46	56.48	0.59	
hincup 3623	4365.0*	2000.00	3899.43	3904.64	5.21	3905.51	3904.530	7.51	266.37	50.56	0.63	
hincup 3623	4365.0*	3500.00	3899.43	3905.06	6.43	3904.91	3907.44	0.006303	10.13	384.10	162.80	0.78
hincup 3623	4365.0*	4082.00	3899.43	3906.18	6.75	3906.18	3908.00	0.006841	10.96	448.07	207.88	0.80
hincup 3623	4365.0*	5529.00	3899.43	3907.13	7.70	3907.22	3909.11	0.006576	11.88	645.86	211.76	0.80
hincup 3623	4365.0*	6326.00	3899.43	3907.22	7.79	3907.55	3909.70	0.006098	13.31	685.94	212.17	0.90

TEC-RAS Plan: w/ bridges 3 River: lin cup Reach: lincup (Continued)

Reach	River Sta	Q Total	Min Ch El	W.S. Elev	Max Ch Depth	Crit W.S.	E.G. Elav	E.G. Slope	Vel Chnl	Flow Area	Top Width	Act Froude # Chi
		(cfs)	(ft)	(ft)	(ft)	(ft)	(ft)	(ft)	(ft/s)	(sq ft)	(ft)	
lincup	3208*	160.00	3898.65	3899.96	1.31		3900.06	0.000461	2.73	68.52	50.45	0.45
lincup	3208*	1355.00	3808.65	3902.78	4.13		3903.41	0.004266	6.36	213.19	59.11	0.59
lincup	3208*	2000.00	3998.65	3903.71	5.06		3904.56	0.004544	7.42	276.41	148.78	0.63
lincup	3208*	3500.00	3898.65	3904.65	6.00		3906.06	0.006331	9.95	484.10	239.74	0.76
lincup	3208*	4082.00	3898.65	3904.95	8.30		3905.16	0.008689	10.62	540.22	284.75	0.79
lincup	3208*	5529.00	3898.65	3905.33	6.68		3905.89	0.009820	12.81	642.03	268.39	0.82
lincup	3208*	6326.00	3898.65	3905.75	7.10		3906.21	0.008342	12.96	755.68	269.80	0.90
lincup	3000.5*	160.00	3897.88	3899.30	1.42		3899.40	0.000361	2.57	62.32	53.88	0.42
lincup	3000.5*	1355.00	3897.88	3901.67	3.79		3902.39	0.005618	6.84	198.25	62.01	0.67
lincup	3000.5*	2000.00	3897.88	3902.05	4.17		3903.28	0.008380	8.93	244.43	180.83	0.83
lincup	3000.5*	3500.00	3897.88	3903.06	5.18		3903.40	0.008481	10.57	474.62	266.05	0.87
lincup	3000.5*	4082.00	3897.88	3903.31	6.43		3903.70	0.008666	11.20	543.95	284.82	0.90
lincup	3000.5*	5529.00	3897.88	3904.04	6.16		3904.35	0.007922	11.62	770.23	327.65	0.87
lincup	3000.5*	6326.00	3897.88	3904.50	6.71		3905.91	0.008260	10.99	850.91	329.32	0.78
lincup	2793	160.00	3897.10	3898.46	1.36		3898.60	0.00027	2.92	54.79	56.76	0.52
lincup	2793	1355.00	3897.10	3900.53	3.73		3901.30	0.004287	5.92	316.41	250.01	0.58
lincup	2793	2000.00	3897.10	3901.38	4.28		3901.88	0.004113	6.44	484.33	288.80	0.59
lincup	2793	3500.00	3897.10	3902.87	5.77		3903.20	0.002147	5.82	971.68	387.37	0.45
lincup	2793	4082.00	3897.10	3903.39	6.29		3903.68	0.001710	5.65	1176.36	388.88	0.41
lincup	2793	5529.00	3897.10	3904.18	7.08		3904.48	0.001611	5.85	1462.29	391.13	0.40
lincup	2793	6326.00	3897.10	3904.79	7.69		3905.08	0.001344	5.68	1723.26	392.80	0.37
lincup	2592*	160.00	3896.20	3897.45	1.25		3897.58	0.005031	2.95	54.27	55.45	0.53
lincup	2592*	1355.00	3896.20	3899.91	3.71		3900.43	0.004346	6.08	306.50	246.27	0.58
lincup	2592*	2000.00	3896.20	3900.51	4.31		3901.04	0.004120	6.61	485.34	291.00	0.59
lincup	2592*	3500.00	3896.20	3902.63	6.43		3902.84	0.001219	4.81	1188.42	354.86	0.35
lincup	2592*	4082.00	3896.20	3903.18	6.98		3903.39	0.001056	4.75	1386.42	356.57	0.33
lincup	2592*	5529.00	3896.20	3903.95	7.75		3904.20	0.001128	5.28	1690.67	358.91	0.34
lincup	2592*	6326.00	3896.20	3904.59	8.39		3904.83	0.000993	5.24	1891.40	360.88	0.33
lincup	2391*	160.00	3895.30	3896.46	1.18		3896.61	0.004784	2.93	54.63	54.25	0.51
lincup	2391*	1355.00	3895.30	3899.00	3.70		3899.55	0.004372	6.20	297.64	246.03	0.59
lincup	2391*	2000.00	3895.30	3899.56	4.26		3900.17	0.004485	6.95	446.93	292.15	0.62
lincup	2391*	3500.00	3895.30	3902.48	7.18		3902.63	0.000758	4.14	1373.28	323.44	0.28
lincup	2391*	4082.00	3895.30	3903.04	7.74		3903.20	0.000709	4.22	1568.46	325.35	0.27
lincup	2391*	5529.00	3895.30	3903.78	8.48		3903.99	0.000845	4.91	1796.62	330.04	0.30

HEC-RAS Plan: w/ bridges 3 River: In cup Reach: Inlcup (Continued)

Reach	River Sta	Q Total	Min Ch El	W.S. Elev	Max Ch Depth	Crit W.S.	E.G. Elevation	E.G. Slope	Vel Chnl	Flow Area	Top Water Act	Friction # Chnl
		(cfs)	(ft)	(ft)	(ft)	(ft)	(ft)	(ft/ft)	(ft/s)	(sq ft)	(ft)	
Inlcup	2391. *	6326.00	3894.42	9.12		3904.64	0.000827	5.10	2016.09	353.30	0.30	
Inlcup	2190. *	160.00	3894.40	3895.51	1.11		3895.05	0.004852	2.97	53.83	53.07	
Inlcup	2190. *	1355.00	3894.40	3898.06	3.68		3898.66	0.004478	6.32	288.60	251.43	0.60
Inlcup	2190. *	2000.00	3894.40	3898.79	4.39		3899.33	0.003722	6.54	483.10	279.89	0.57
Inlcup	2190. *	2500.00	3894.40	3902.36	7.96		3902.50	0.000573	3.89	1537.52	341.00	0.25
Inlcup	2190. *	4082.00	3894.40	3902.93	8.53		3903.07	0.000556	4.01	1736.44	351.66	0.25
Inlcup	2190. *	5529.00	3894.40	3903.54	9.24		3903.83	0.000682	4.70	1987.51	353.90	0.28
Inlcup	2190. *	6326.00	3894.40	3904.29	9.90		3904.49	0.000639	4.77	2220.09	355.97	0.27
Inlcup	1989. *	160.00	3893.50	3894.65	1.15		3894.77	0.003918	2.80	57.15	52.20	0.47
Inlcup	1989. *	1355.00	3893.50	3897.56	4.06		3897.92	0.002591	5.19	382.88	245.56	0.47
Inlcup	1989. *	2000.00	3893.50	3898.37	4.87		3898.71	0.002161	5.39	682.64	251.80	0.44
Inlcup	1989. *	3500.00	3893.50	3902.30	8.80		3902.40	0.000346	3.24	1738.85	302.14	0.20
Inlcup	1989. *	4082.00	3893.50	3902.87	9.37		3902.97	0.000392	3.42	1909.86	302.14	0.20
Inlcup	1989. *	5529.00	3893.50	3903.55	10.05		3903.71	0.000470	4.14	2117.36	302.14	0.23
Inlcup	1989. *	6326.00	3893.50	3904.21	10.71		3904.37	0.000466	4.30	2314.68	302.14	0.23
Inlcup	1788. *	160.00	3892.60	3894.36	1.76		3894.41	0.000938	1.81	88.58	52.98	0.25
Inlcup	1788. *	1355.00	3892.60	3897.39	4.79		3897.55	0.001050	3.72	564.64	246.00	0.31
Inlcup	1788. *	2000.00	3892.60	3898.19	5.59		3898.37	0.001031	4.11	762.29	246.00	0.31
Inlcup	1788. *	3500.00	3892.60	3902.25	9.65		3902.33	0.000954	3.02	1759.87	246.00	0.17
Inlcup	1788. *	4082.00	3892.60	3902.81	10.21		3902.91	0.000985	3.26	1897.82	246.00	0.18
Inlcup	1788. *	5529.00	3892.60	3903.46	10.86		3903.62	0.000405	4.06	2059.44	246.00	0.22
Inlcup	1788. *	6326.00	3892.60	3904.11	11.51		3904.28	0.000421	4.30	2217.87	246.00	0.23
Inlcup	1788. *	160.00	3892.40	3893.64	1.24		3893.87	0.0008392	3.83	41.80	42.47	0.68
Inlcup	1542. *	1355.00	3892.40	3895.05	4.46		3897.16	0.002355	4.98	480.76	255.78	0.45
Inlcup	1542. *	2000.00	3892.40	3897.72	5.32		3898.01	0.002018	5.27	769.79	342.90	0.43
Inlcup	1542. *	3500.00	3892.40	3902.19	9.79		3902.26	0.000280	3.07	2292.71	342.90	0.18
Inlcup	1542. *	4082.00	3892.40	3902.75	10.35		3902.33	0.000298	3.29	2484.17	342.90	0.19
Inlcup	1542. *	5529.00	3892.40	3903.36	10.98		3903.51	0.000422	4.09	2701.57	342.90	0.22
Inlcup	1542. *	6326.00	3892.40	3904.02	11.62		3904.17	0.000433	4.31	2922.67	342.90	0.23
Inlcup	1446.33*	160.00	3891.63	3892.82	1.19		3893.06	0.000384	3.95	40.52	39.09	0.68
Inlcup	1446.33*	1355.00	3891.63	3898.54	4.91		3898.92	0.002418	5.35	419.19	288.16	0.46
Inlcup	1446.33*	2000.00	3891.63	3897.52	5.89		3897.83	0.001774	5.26	703.86	291.93	0.41
Inlcup	1446.33*	3500.00	3891.63	3902.15	10.52		3902.23	0.000277	3.18	2054.96	291.93	0.18
Inlcup	1446.33*	4082.00	3891.63	3902.70	11.07		3902.80	0.000300	3.43	2218.80	291.93	0.19

HEC-RAS Plan: w/ bridges J River: Inlet Reach: Inlet (Continued)

Reach	River Sta.	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Max Ch Dpth (ft)	Ch W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width Act (ft)	Froude # Chnl
Inlet	1446.33*	5529.00	3891.63	3903.32	11.69		3903.47	0.000434	4.28	2395.35	291.93	0.23
Inlet	1446.33*	6326.00	3891.63	3903.95	12.32		3904.12	0.000451	4.54	2582.02	291.93	0.23
Inlet	1350.66*	160.00	3890.87	3892.12	1.25		3892.35	0.006680	3.78	42.28	36.55	0.62
Inlet	1350.66*	1355.00	3890.87	3895.33	5.46		3895.71	0.002662	5.23	387.95	240.97	0.43
Inlet	1350.66*	2000.00	3890.87	3897.34	6.47		3897.67	0.001613	5.27	630.75	240.97	0.39
Inlet	1350.66*	3500.00	3890.87	3902.11	11.24		3902.21	0.000277	3.28	1779.42	240.97	0.18
Inlet	1350.66*	4082.00	3890.87	3902.66	11.79		3902.77	0.000303	3.55	1911.49	240.97	0.19
Inlet	1350.66*	5529.00	3890.87	3903.24	12.37		3903.42	0.000448	4.46	2052.92	240.97	0.23
Inlet	1350.66*	6326.00	3890.87	3903.88	13.01		3904.07	0.000471	4.74	2205.35	240.97	0.24
Inlet	1255.00	160.00	3890.10	3891.82	1.72		3891.05	0.002545	2.86	55.85	35.05	0.40
Inlet	1255.00	1355.00	3890.10	3895.89	5.79		3895.83	0.002143	6.07	226.01	55.00	0.49
Inlet	1255.00	2000.00	3890.10	3896.31	6.21		3896.35	0.004499	8.22	249.04	55.00	0.63
Inlet	1255.00	4082.00	3890.10	3902.57	12.47							
Inlet	1255.00	5529.00	3890.10	3903.10	13.00							
Inlet	1255.00	6326.00	3890.10	3903.72	13.61							
Inlet	1185.00											
Inlet	1100	160.00	3899.40	3891.17	1.77		3890.44	0.003133	0.033105	3.19	50.22	0.44
Inlet	1100	1355.00		3895.44								
Inlet	1100	2000.00		3895.95								
Inlet	1100	4082.00	3889.40	3897.09	7.70		3896.55	0.005002	10.38	633.47		
Inlet	1100	5529.00	3889.40	3898.03	8.63		3898.59	0.002646	8.23	1327.09		
Inlet	1100	6326.00	3889.40	3898.40	9.00		3898.91	0.002761	8.66	1434.66	291.60	
Inlet	891.666*	160.00	3888.47	3890.61	2.14		3890.76	0.002383	3.05	52.43	28.27	0.39
Inlet	891.666*	1355.00	3888.47	3895.00	6.53		3892.92	0.001976	5.61	368.70		
Inlet	891.666*	2000.00	3888.47	3895.72	7.25		3894.58	0.001408	5.14	827.68		
Inlet	891.666*	3500.00	3888.47	3896.51	8.04		3895.51	0.002123	6.84	1088.25	329.10	0.46
Inlet	891.666*	4082.00	3888.47	3896.01	8.34		3895.73	0.002280	7.29	1186.43	329.10	0.48
Inlet	891.666*	5529.00	3888.47	3897.52	9.05		3898.03	0.002527	8.16	1418.79	329.10	0.51
Inlet	891.666*	6326.00	3888.47	3897.86	9.39		3896.46	0.002644	8.59	1533.37	329.10	
Inlet	891.666*	160.00	3887.53	3890.16	2.63		3899.01	0.002014	3.00	53.36	25.86	0.37
Inlet	891.666*	1355.00	3887.53	3894.68	7.15		3892.67	0.001581	5.19	463.23	200.00	0.38

SECTIONS GEOMETRIC DATA
AND MEASUREMENTS OF STRUCTURE

HEC-RAS Plan: w/ bridges 3 River: tin cup Reach: tincup (Continued)

Reach	River Sta	Q Total (cfs)	Min Chd (ft)	W.S. Elev (ft)	Max Chd Depth (ft)	Chd W.S. (ft)	E.G. Slope	Vel Chd (ft/s)	Flow Area (sq ft)	Top Water Act (ft)	Froude # Chd
tincup	683.333*	2000.00	3887.53	3884.86	7.35	3894.24	3895.47	0.00283.9	7.14	504.39	0.51
tincup	683.333*	3500.00	3887.53	3886.09	6.56	3895.13	3896.47	0.002041	6.82	1158.30	0.45
tincup	683.333*	4082.00	3887.53	3886.35	8.82	3895.34	3896.77	0.002238	7.32	1253.62	0.47
tincup	683.333*	5529.00	3887.53	3887.00	9.47	3895.82	3897.50	0.002511	8.20	1493.76	0.51
tincup	683.333*	6326.00	3887.53	3887.33	9.80	3896.08	3897.86	0.002635	8.62	1613.06	0.52
tincup	475	160.00	3886.60	3888.57	1.97	3888.57	3889.21	0.017564	6.43	24.87	1.01
tincup	475	1355.00	3886.60	3882.69	6.09	3882.69	3894.05	0.008572	9.62	180.74	0.81
tincup	475	2000.00	3886.60	3883.91	7.31	3883.91	3894.69	0.004445	8.33	528.35	0.62
tincup	475	3500.00	3886.60	3894.71	8.11	3894.71	3895.65	0.005583	10.13	827.08	0.71
tincup	475	4082.00	3886.60	3895.02	8.42	3895.02	3895.94	0.005575	10.36	952.56	0.71
tincup	475	5529.00	3886.60	3895.47	8.87	3895.47	3896.54	0.006189	11.72	1133.30	0.77
tincup	475	6326.00	3886.60	3895.68	9.06	3895.68	3896.84	0.006684	12.41	1217.75	0.81
tincup	181.7	160.00	3884.40	3886.81	2.41	3885.66	3886.94	0.001968	2.95	54.15	0.36
tincup	181.7	1355.00	3884.40	3882.10	7.70	3889.83	3892.20	0.000521	3.24	901.13	0.22
tincup	181.7	2000.00	3884.40	3882.84	8.44	3890.59	3892.96	0.000622	3.80	1139.17	0.25
tincup	181.7	3500.00	3884.40	3893.79	9.39	3891.47	3893.98	0.000966	5.13	1449.16	0.31
tincup	181.7	4082.00	3884.40	3894.08	9.69	3891.72	3894.31	0.001087	5.57	1546.56	0.33
tincup	181.7	5529.00	3884.40	3894.73	10.33	3892.32	3895.03	0.001369	6.55	1757.08	0.38
tincup	181.7	6326.00	3884.40	3895.04	10.64	3892.67	3895.39	0.001514	7.05	1869.19	0.40
tincup	154	Bridge	<u>WATER STREET BRICKES</u>								
tincup	114	160.00	3884.40	3886.62	2.22	3886.78	3886.04	0.002604	3.25	49.29	0.41
tincup	114	1355.00	3884.40	3890.42	6.02	3890.91	3892.18	0.002918	6.31	398.69	0.50
tincup	114	2000.00	3884.40	3891.15	6.75	3891.63	3892.85	0.002805	6.78	609.84	0.50
tincup	114	3500.00	3884.40	3892.24	7.84	3892.79	3893.103	0.003103	8.01	943.60	0.54
tincup	114	4082.00	3884.40	3892.58	8.18	3893.17	3893.185	0.003185	8.39	1055.74	0.55
tincup	114	5529.00	3884.40	3893.36	8.96	3893.99	3893.223	0.003223	9.04	1309.37	0.57
tincup	114	6326.00	3884.40	3893.71	9.31	3894.38	3893.322	0.003322	9.45	1423.53	0.58
tincup	000	160.00	3883.10	3885.14	3.03	3885.44	3886.39	0.004508	4.04	39.57	0.54
tincup	000	1355.00	3883.10	3889.79	6.69	3889.56	3890.48	0.004502	7.43	319.75	210.51
tincup	000	2000.00	3883.10	3890.48	7.38	3890.20	3891.21	0.004501	8.18	466.25	
tincup	000	3500.00	3883.10	3891.58	8.48	3891.22	3892.36	0.004502	9.30	810.84	
tincup	000	4082.00	3883.10	3891.94	8.84	3891.48	3892.73	0.004501	9.64	919.92	
tincup	000	5529.00	3883.10	3892.70	9.60	3892.07	3893.55	0.004503	10.37	1165.99	
	000	6326.00	3883.10	3893.06	9.96		3892.34				

SECTION D-13

FEARON

ASSOCIATES

REDUCED CROSS SECTION NOTES

Project Title:
Engineer's Pre-
Survey Date:

Project No. _____

Tin Carp - Stream Sections

STATION /
LOCATION

36 + 23

Small Wood Bridge

171
125
110
105
100

SMALL WOOD BRIDGE

TB

97	97	97
97	97	97
97	97	97

SEC 3

3500
7526-7

DESC. |

<u>28862</u>	<u>95 4</u>	<u>95 4</u>	<u>92 6</u>	<u>92 6</u>
<u>0</u>	<u>80</u>	<u>100</u>	<u>104</u>	<u>152</u>
<u>0</u>				

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EL. DIST.

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150

DESC.

DIST. I

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DIST.

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DIST.

130

REDUCED CROSS SECTION NOTES

Project Title:

Engineer's Drawing No.

Current Date

REDUCED CROSS SECTION NOTES

Project Title:

Project No. _____

Survey Dates

Job Title Tin Cup Dam By _____ Date _____ Job No. _____Subject INUNDATION STUDY Checked _____ Sheet _____ of _____**SPILLWAY WITH 2' RISE**

CONSIDER BREACH FLOWS FOR RESERVOIR ELEVATION

OF 6286.3, APPROX. CAPACITY OF RESERVOIR = 1134 ACRE-FT.

BREACH FLOW @ DAM = 10049 CFS

BREACH FLOW @ DARBY (REACH #8) = 6326 CFS

From DNRC Model

by L. Siroky

TIME TO EMPTY RESERVOIR

$$t_1 = \frac{1134 \text{ Acre-Ft} \times 43560 \text{ ft}^2/\text{acre}}{10049 \text{ ft}^3/\text{sec}}$$

$$t_1 = 4915.6 \text{ sec} \Rightarrow 82 \text{ min}$$

BECAUSE the highway bridge over Tin Cup Creek is not located directly in the path of the breach flow as it exits the confining valley just above Darby we assume that the highway bridge will remain in place and will be the controlling element in determining the inundation area U/S of Highway 93. From HEC-RAS Modelling the hydraulic capacity of the bridge is estimated to be 3500 cfs when depth of flow is restricted to just below the bridge finish grade elevation, i.e. structure and highway are not overtopped. To determine the inundation boundary U/S of the bridge we assume what does not pass thru the bridge must be stored in the flood plain above the structure.

$$\begin{aligned} Q_{\text{STORE}} &= 6326 \text{ cfs} - 3500 \text{ cfs} \\ &= 2826 \text{ cfs} \end{aligned}$$

$$Var_{\text{STORE}} = 2826 \text{ cfs} \times 82 \text{ min} \times \frac{60 \text{ sec}}{1 \text{ min}}$$

$$Var_{\text{STORE}} = 13,903,920 \text{ ft}^3 \Rightarrow 319.2 \text{ acre-ft}$$

Job Title Tin Cup Dam By _____ Date _____ Job No. _____Subject Inundation Study Checked _____ Sheet _____ of _____

$$V_{STORE} = 319.2 \text{ ACRE - FT}$$

BASED ON CROSS SECTION DATA AND FIELD REVIEW, INITIALLY
ASSUME AN AVERAGE DEPTH OF 3'

$$A_{STORE} = \frac{319.2 \text{ Acre - Ft}}{3} = 106.4 \text{ Acres}$$

ESTIMATE ACTUAL SURFACE
AREA WITHIN INUNDATION AREA
TO BE APPROX 85 ACRES

$$d_{REPO} = \frac{319.2 \text{ Acre - Ft}}{85 \text{ Acre}} = 3.76$$

SPILLWAY WITHOUT 2' RISE
(NORMAL FULL POOL)

CHECK FOR SMALLER FLOW WITH
RESERVOIR ELEVATION OF 6284.3

TIME TO EMPTY RESERVOIR

$$t = \frac{940 \text{ ACRE-FT} \times 43560 \frac{\text{ft}^3}{\text{acre}}}{7353 \text{ CFS}}$$

$$t = 5568.7 \text{ SEC} \Rightarrow 93 \text{ min}$$

$$\begin{aligned} Q_{SOES} &= 5255 \text{ CFS} - 3500 \text{ CFS} \\ &= 1755 \text{ CFS} \end{aligned}$$

$$\begin{aligned} V_{STORE} &= 1755 \text{ CFS} \times 93 \text{ min} \times \frac{60 \text{ SEC}}{\text{min}} \\ &= 9,792,900 \text{ ft}^3 \\ &\text{OR } 224.8 \text{ acre-ft.} \end{aligned}$$

(Approx 70% of $V_{6284.3}$)